Published in final edited form as:

Adv Civ Eng Mater. 2018 ; 7(1): 71–86. doi:10.1520/ACEM20170126.

Relating the Electrical Resistance of Fresh Concrete to Mixture Proportions

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Abstract

Characterization of fresh concrete is critical for assuring the quality of our nation's constructed infrastructure. While fresh concrete arriving at a job site in a ready-mixed concrete truck is typically characterized by measuring temperature, slump, unit weight, and air content, here the measurement of the electrical resistance of a freshly cast cylinder of concrete is investigated as a means of assessing mixture proportions, specifically cement and water contents. Both cement and water contents influence the measured electrical resistance of a sample of fresh concrete: the cement by producing ions (chiefly K^+ , Na^+ , and OH^-) that are the main source of electrical conduction; and the water by providing the main conductive pathways through which the current travels. Relating the measured electrical resistance to attributes of the mixture proportions, such as water-cement ratio by mass (w/c), is explored for a set of eleven different concrete mixtures prepared in the laboratory. In these mixtures, w/c , paste content, air content, fly ash content, high range water reducer dosage, and cement alkali content are all varied. Additionally, concrete electrical resistance data is supplemented by measuring the resistivity of its component pore solution obtained from 5 laboratory-prepared cement pastes with the same proportions as their corresponding concrete mixtures. Only measuring the concrete electrical resistance can provide a prediction of the mixture's paste content or the product w^*c ; conversely, when pore solution resistivity is also available, w/c and water content of the concrete mixture can be reasonably assessed.

Keywords

Cement content; electrical resistance; formation factor; mixture proportions; paste content; porosity; resistivity; water-cement ratio; water content

Introduction

Performance-based quality control of concrete is an important objective of the ready-mixed concrete industry and its customers including engineers and building owners. To assure fulfillment of its intended function, job-site measurements of concrete temperature, slump, density (unit weight), and air content are routinely performed following prescribed ASTM International standard test methods. Taken together, measurements of air content and density provide some indication that the mixture proportions of the concrete coming from the ready-mixed concrete truck are the same as the ones detailed in the job specifications.

Other tests to confirm the mixture proportions have long been sought, such as a microwave field test to estimate the water content of the delivered concrete [1-3]. As cementitious content is typically known from the batch tickets, the water-cement ratio (w/c) or water-cementitious materials ratio (w/cm) by mass could then be estimated. More recently, Mancio et al. [4] have shown the potential of using measurements of electrical resistivity to estimate w/c of concrete. In their study on 8 concrete mixtures designed per the ACI 211.1 procedures, w/c was varied only by changing the cement content (c), while keeping the water content constant. This ensured that the electrical resistivity increased with an increase in w/c , as for a higher w/c (lower cement content), there were fewer conductive ions being released by the reduced quantity of cement (per unit volume of concrete). Importantly, for potential field use of this technology, these authors also showed that “time did not have a statistically significant effect on the electrical resistivity of fresh concrete before initial setting” [4].

Based on this and other research [5-6], in 2015, ASTM subcommittee C09.60 (Testing Fresh Concrete) formed a task group to develop a standard practice for measuring the electrical resistance, R , of fresh concrete. (The draft document is currently under subcommittee balloting.) To support the development of this document, the present study examines the electrical resistance of 11 concrete mixtures, along with the electrical resistivity of 5 of their component pore solutions.

Viewing concrete as a conventional porous media and assuming non-conducting solids (aggregates, cement particles, supplementary cementitious materials (SCMs)), the measured electrical conductivity (σ) or resistivity (ρ) of a fresh concrete sample will be determined by the conductivity of the pore solution (σ_0 ; the major conducting component) and characteristics of the solution-filled porosity including pore volume fraction (ϕ) and tortuosity/connectivity. The ratio of concrete conductivity to pore solution conductivity, or equivalently pore solution resistivity (ρ_0) to concrete resistivity, can be described using an equation of the form [7]:

$$\frac{\sigma}{\sigma_0} = \frac{\rho_0}{\rho} = \frac{1}{F} = a \cdot \phi^n \quad \text{or} \quad \phi = \left(\frac{\rho_0}{a \cdot \rho} \right)^{\frac{1}{n}} \quad (1)$$

where F is the formation factor adapted from rock geology, a is a constant determined by pore geometry, and n typically takes on values between 1 and 2. In a fresh concrete, $\emptyset$ is directly proportional to the water content (w , water mass per unit volume of concrete), as the air voids are neither water-filled nor conductive. In a typical experiment, fresh concrete resistance (R) is measured directly. If desired, resistivity can then be calculated using a geometry factor, K , as

$$\rho = KR \quad (2)$$

In the above analysis, a single and constant temperature is assumed, since the measured resistance/resistivity is also a function of temperature [8].

Figure 1 shows some possible values for n , varying from a lower bound of 1 for a simple parallel model, through a value of 1.5 for a suspension of insulating particles in a 3-D conducting fluid [7], to a value of 2 for a hydrated cement paste (HCP) or concrete specimen [9]. Here, equation (1) will provide a basis for developing relationships between electrical resistance measurements and concrete mixture proportions. A change in mixture proportions will alter both the concrete resistance and the pore solution resistivity, as a lower w/c concrete will have a higher ionic concentration and hence a lower pore solution resistivity.

Research Significance

Assessment and control of the mixture proportions of ready-mixed concrete is a critical step in meeting project specifications and producing a quality structure. Measurements of the electrical resistance of fresh concrete may serve a valuable role in this process. Here, the potential and limitations of this technique are evaluated based on the measured properties of 11 different concrete mixtures prepared in the laboratory. The advantages of supplementing the electrical resistance of the fresh concrete with a measurement of the resistivity of its pore solution to effectively compute a formation factor are also explored.

Objective

The objectives of the present study are as follows:

1. Develop correlation curves between w/c (or w/cm) and measured resistance
 - at constant paste volume –when w/c is varied, both water and cement contents are varied, and
 - at varying paste volumes and constant cement content - when w/c is varied, only water content is varied

The findings will help determine which alternative is more sensitive to a change in resistance. The draft ASTM practice allows for both options.

2. Compare immediately measured resistance values with those obtained after 90 min [4]

- This will provide background on how to interpret field data vs. lab data. Field resistance will typically be measured in the time frame of 45 min to 90 min after mixing, while lab data can be measured immediately after mixing.
3. Investigate the impact of the following variables of concrete mixtures at the same w/c or w/cm on measured electrical resistance:
 - Change in paste volume
 - Use of 25% Class F fly ash by volume in the mixture
 - Variation of the alkali content of the portland cement (i.e, a high-alkali cement)
 - Use of admixtures such as a high range water-reducing admixture (HRWRA)
 - Air entrainment
 4. Develop single operator precision information.
 5. Relate measured electrical resistance to mixture proportions based on equation (1).

Materials and Methods

Materials

The following materials were used in this study:

- ASTM C150 [10] Type I/II portland cement (low alkali) with $Na_2O_{eq} = 0.48\%$
- ASTM C150 [10] Type I/II portland cement (high alkali) with $Na_2O_{eq} = 0.90\%$
- ASTM C618 [11] Class F fly ash
- ASTM C33 [12] No. 57 crushed coarse aggregate
- ASTM C33 [12] natural sand with a fineness modulus, $FM=2.83$
- ASTM C494 [13] Type F polycarboxylate high range water-reducing admixture
- ASTM C260 [14] inorganic air-entraining admixture

Target mixture proportions

To complete the objectives outlined above, an experimental design was developed consisting of 10 unique concrete mixtures. As detailed in Table 1, in Mixtures 1, 2, and 3, the w/c was varied as 0.37, 0.42, and 0.47, respectively. The quantity of portland cement content was maintained constant and the mixing water content was varied, thus allowing the paste volume to vary. In mixtures 2, 4, and 5 the w/c was again varied as 0.42, 0.37, and 0.47, respectively, but while maintaining the same paste volume. Mixture 6 had the same w/c as Mixture 3, but was proportioned at a lower paste volume. Mixture 7 included 25% fly ash by volume of cementitious materials, with the same w/cm and paste volume as Mixture 2.

Mixture 8 had the same w/c and paste volume as Mixture 2, but was produced using a high alkali cement with an equivalent alkali content of 0.90%. Mixture 9 had the same w/c and paste volume as Mixture 3, but the HRWRA was not used in this mixture. Mixture 10 is an air-entrained mixture with the same w/c and paste volume as Mixture 2. Due to an error in the preparation of Mixture 4, it had to be recast as Mixture 4R, hence, the total of eleven mixtures investigated in the present study.

The mixtures were all non-air-entrained except for Mixture 10. The properties measured on the fresh concrete included slump (ASTM C143 [15]), air content using both the gravimetric and pressure test methods (ASTM C138 [16] and ASTM C231 [17]), temperature (ASTM C1064 [18]), and density (ASTM C138 [16]), and two 4" $\times$ 8" (100 mm by 200 mm) cylinders were prepared for measuring compressive strength at 42 d (ASTM C39 [19]). Cylinders for compressive strength testing were demolded after 1 d and stored until the age of testing in a fog room maintained at 23°C $\pm$ 2°C.

The following procedures were used on Mixtures 1-3 to obtain information on single laboratory repeatability of the resistance measurements and to evaluate the impact of elapsed time. After the concrete was mixed following standard laboratory procedures [20], a sample of concrete was obtained for the fresh and hardened properties, while the remaining concrete was retained in the mixer. Then, six nominally 4" $\times$ 8" (100 mm by 200 mm) concrete cylinders were prepared for the resistance measurements following ASTM Standard Practice C192/C192M [20]. An electrode assembly was carefully inserted into the filled mold, any excess concrete was carefully removed from the top surface, the top surface was sealed with a lid and the outside of the mold was tapped lightly 10 to 15 times using a wooden mallet. The resistance measurements were made on each cylinder by two operators using two sets of devices resulting in 12 resistance measurements for each mixture. The operators interchanged cylinders for measurements. These "initial" measurements of resistance were all conducted within 14 min to 48 min after the time of first contact between cement and water. One cylinder was left connected to a measuring device to obtain a resistance measurement at 90 min. The concrete left in the mixer was covered to avoid evaporation and (re)mixed for 1 min every 5 min until about 90 min. A sample was obtained at 90 min from which three cylinders were prepared for resistance measurements. These resistance measurements were made between 86 min and 109 min. For mixtures 4-10, three concrete cylinders were prepared for the resistance measurements and the resistance was measured by only one operator. No resistance measurements were made beyond 90 min on any of the concrete mixtures.

Fresh and hardened concrete measurements at the National Ready-Mixed Concrete Association (NRMCA)

Concrete mixture proportions and test results are provided in Table 2. Since Mixture 4 had been cast with an incorrect w/c of 0.39, Mixture 4R had to be cast with the correct w/c of 0.37. For all the mixtures, the measured slump varied between 2 in. (50 mm) and 9 in. (225 mm). HRWRA dosages (Table 2) were adjusted to keep the slump within this range. All concrete mixtures were readily consolidated into their cylinder molds, as certainly, improper or incomplete consolidation would surely influence the subsequent resistance measurements.

Measurement of electrical resistance of fresh concrete

The wireless device used in this study for the measurement of electrical resistance of concrete is the SmartBox apparatus manufactured by Giatec¹ and is shown in Figure 2. An alternating current (AC) is applied between the two electrodes inserted in the specimen, and the voltage is measured concurrently. The electrical resistance is calculated from the ratio of the measured voltage to the applied current and is directly reported by the device. In this study, the electrical resistance is measured on a 4" × 8" (100 mm by 200 mm) cylindrical specimen of the freshly mixed concrete. Using 0.5% and 1% by mass NaCl calibration solutions with reported resistivities of 1.22 Ω·m and 0.625 Ω·m, respectively, at 20°C [21], a geometry factor, K , of 0.12 m±0.1 m was determined for the experimental setup shown in Figure 2. However, for the measurements performed on fresh concrete mixtures in this study, only resistance data will be reported and analyzed, as these values were reported directly by the device and readily available to the end user.

Pore solution evaluation at the National Institute of Standards and Technology (NIST)

Using the same low-alkali cement, fly ash, and HRWRA as employed for the concrete mixtures at NRMCA, 5 cement pastes were prepared at NIST. For the paste with 25% fly ash, the cement and fly ash were preblended dry for 30 min in a 3D mixer that allows the material to be tumbled and rolled simultaneously. Three of the pastes were prepared with a w/c of 0.37, 0.42, and 0.47, respectively, and a HRWRA dosage matching those of concrete mixtures 1, 2, and 3 in Table 2. The paste mixture with fly ash matched concrete Mixture 7 in Table 2. Finally, a $w/c=0.42$ mixture without HRWRA was also prepared.

Pastes were prepared in a temperature-controlled high shear mixer following the procedures outlined in ASTM C1738 [22]. The cooling water bath attached to the mixing container was set at 15°C to obtain a paste temperature after mixing of between 21.5°C and 23.5°C. Pastes were sampled, at 30 min±5 min after mixing, into two balanced centrifuge tubes (about 25 g of paste in each). These samples were centrifuged at 4000 rpm (420 rad/s) for 3 min. The pore solution was carefully removed from the centrifuge tube. If the extracted solution was not transparent, a second round of centrifuging was performed, balancing its tube with an equally filled tube of water. The extracted pore solution was then placed in a small capillary (cylindrical) cell and its resistance measured using a commercial impedance analyzer housed in a walk-in environmental chamber maintained at 25°C±1°C. Calibration was performed by measuring the impedance response of a 0.1 mol/L solution of KCl with a reference resistivity value of 0.78 Ω·m at 25°C [23]. Using the measured response of this solution, the computed K value of 4420 m for the capillary cell was employed to convert the measured electrical resistances of the pore solutions in units of Ω to their corresponding resistivity values in units of Ω·m. Two to four resistivity measurements were made for each extracted solution.

¹Certain commercial products are identified in this paper to specify the materials used and the procedures employed. In no case does such identification imply endorsement or recommendation by NIST or NRMCA, nor does it indicate that the products are necessarily the best available for the purpose.

Results

Paste pore solution resistivity results

The measured pore solution resistivities are plotted against the paste w/c in Figure 3, along with the results from the calibration regression. The $w/cm=0.42$ mixture with 25% fly ash by volume corresponds to a $w/c=0.535$, the rightmost data point in the figure. A strong linear relationship between w/c and measured resistivity is observed, with w/c within the range of 0.35 to 0.55 being predictable to within about ± 0.015 via this electrical measurement, per a calibration regression analysis [24] performed in the statistical computing package R, using a Monte Carlo simulation with 10,000 trials conducted at each simulated value of resistance (0.015 representing the average uncertainty in the predicted w/c for resistivities in the range of $0.20\Omega\cdot m$ to $0.30\Omega\cdot m$). One would expect the pore solution conductivity to be proportional to the number of charge carriers per unit volume of solution [25], implying a direct linear proportionality between c/w and conductivity, with different coefficients for each individual cement (binder). This would be equivalent to the observed linear relationship between their respective inverses, w/c and resistivity, observed in Figure 3. The overlapping of the two data points for $w/c=0.42$ indicates that the presence or absence of the HRWRA did not significantly influence the measured pore solution resistivity for the materials employed in this study. The values in Figure 3 are in reasonable agreement with those predicted by the NIST pore solution conductivity model based on the cement composition (equivalent alkalis) and mixture proportions [25,26], if it is assumed that about 55% of the alkalis present in the cement are readily soluble upon contact with water (75% is the default assumption in the NIST model) and that the alkalis provided by the fly ash used in this study are negligible.

Relating electrical resistance of fresh concrete to mixture proportions

Concrete measurements only—In Table 2, the resistance measurements of Mixtures 1-3 are an average of 12 readings, while the resistance measurements on Mixtures 4-10 are an average of 3 readings. The complete set of raw data as well as a greater discussion of the results can be found in Obla et al. [27]. The following observations can be made.

Figure 4 shows a correlation between measured resistance, compressive strength, and w/c for Mixtures 1, 2, and 3. For these three mixtures, a reduction in w/c corresponded with a reduction in mixing water content that resulted in fewer pathways for the transport of the charged ions and therefore increased resistance. Having fewer pathways overcame the reduction in pore solution resistivity (Figure 3) when the w/c was decreased. A reduction in w/c from 0.47 to 0.37 resulted in a small increase in measured resistance of about 18% and a much higher increase of about 51% in 42d compressive strength. For the data shown in Figure 4, linear fits to both resistance and compressive strength vs. w/c yielded coefficients of determination (R^2) that were very close to 1.

Figure 5 shows a similar plot of measured resistance and compressive strength vs. w/c for Mixtures 4R, 4, 2, and 5. Since paste volume is kept constant for these mixtures, as the w/c decreased, the water content decreased, while cement content also increased. This resulted in fewer pathways for more charged ions. These two opposing factors resulted in only a small decrease in the measured resistance as w/c was decreased. This suggests that when

developing a laboratory correlation between resistance and w/c , maintaining a constant paste volume may produce less of a change in electrical resistance than when the paste volume is varied. Strength is a better predictor of w/c than measured resistance for these mixtures, although it was not measured until 42 d after casting and therefore could not be employed as an onsite immediate quality control procedure. For the data shown in Figure 5, while the coefficient of determination remains at 0.98 for the compressive strength data, it is only 0.14 for the resistance data.

Reducing the paste volume (Mixture 6) while maintaining the same w/c as in Mixture 2 resulted in a 42% increase in measured resistance. Including 25% Class F fly ash by volume of cementitious materials (Mixture 7) while maintaining the same w/cm as in Mixture 2 resulted in a 25% higher measured resistance. In both cases, the higher measured resistance was likely due to the reduced cement content that contributed less charged ions to the pore solution. Additionally, in the first case, the lower mixing water content also contributed to the increased resistance.

Using a high alkali cement (Mixture 8), while maintaining the same w/c as in Mixture 2, resulted in a 35% lower measured resistance. Unfortunately, the same sample of high alkali cement was not available for pore solution resistivity testing. In comparison, the NIST pore solution conductivity model [25,26] predicts that the high alkali cement mixture would have a 43% lower pore solution resistivity than the low alkali cement mixture, if the extent of soluble alkalis were the same for both. Neither the non-use of the HRWRA (Mixture 9) nor the use of air-entrainment (Mixture 10) significantly influenced the measured resistance. Readings after 90 min of simulated mixing are higher than those taken at 90 min on initially cast specimens for the lowest w/c , but similar for the two higher w/c . Additionally, the readings taken at 90 min on initially cast specimens were within 4% of their original values.

The single operator standard deviation for the resistance measurement was determined to be 1.4Ω , based on the worst case (Mixture 1). The single-operator coefficient of variation was 3.9%. The average resistance of the mixtures evaluated varied between 24Ω and 47Ω . Therefore, the results of two properly conducted tests by the same operator on specimens prepared from the same sample of concrete are not expected to differ by more than 4Ω or 10.8% of the average in more than 95% of the cases. Based on the precision statement of the slump test and the specified tolerances for slump in ASTM C94 [28], it seems appropriate to suggest a tolerance of $\pm 4\Omega$ to the specified value if this test were to be used as an acceptance test. If these tolerances are overlaid on average data from Mixtures 1, 2, and 3, it will create considerable overlap, suggesting that it would be difficult to distinguish between 0.37 and 0.47 w/c concrete mixtures using a single resistance measurement. So, even if the only variable between different batches is the mixing water content as in Mixtures 1-3, the precision and the sensitivity of the test needs to be improved before it could be used as a reliable estimator for the w/c or w/cm .

Figure 6 shows that the measured fresh concrete electrical resistance has basically no correlation with w/c (w/cm) when variables such as paste volume, dosage of SCM, and cement alkali content are varied. Figure 7 shows the inverse of resistance (or conductance) for all of the mixtures containing low alkali cement plotted against either the volume of

cement or the volume of water or the volume of cement and water in a cubic yard of concrete. An increase in either cement or water volume led to a decrease in resistance (more ions and more pathways, respectively), however, the measured resistance correlated best with the sum of cement and water volumes.

Returning to equations (1) and (2), the measured concrete resistance should be proportional to the resistivity of the pore solution divided by (the water-filled porosity raised to the power of n) or (ρ_0/w^n) . Since Fig. 3 shows that pore solution resistivity is proportional to w/c , one might expect that the concrete resistance would then be proportional to $1/(c \cdot w^{n-1})$. For $n=1$, the proportionality would simply be to $1/c$, proportional to the volume of cement fit shown by the leftmost data set in Figure 7. For $n=2$, the proportionality would be to $1/(c \cdot w)$, while for $n=1.5$ (spherical inclusion case in Figure 1), it would be to $1/(c \cdot w^{0.5})$. Scaled versions of these latter two relationships are plotted in Figure 8, along with one based on the inverse of the (cement+water) volume, or $1/(w+c/3.15)$, where 3.15 represents the specific gravity of the cement used in this study. Linear fits with a coefficient of determination greater than 0.9 are obtained for relating resistance to the inverse of $c \cdot w^{n-1}$ for either $n=1.5$ or 2, while a coefficient of determination of 0.722 was obtained for $n=1$ in Figure 7. This suggests that the measured electrical resistance of fresh concrete can potentially provide information on the product of cement and water contents, or perhaps their sum (as indicated by the (cement+water) volume in Fig. 7 and by the $3000/(w+c/3.15)$ scaled paste volume data in Figure 8, although it does not uniquely relate to w/c or either w or c separately. The relationships shown in Figure 8 would only be valid for concretes prepared with the low alkali cement and aggregates employed in this study, as any change in materials would require additional measurements to develop a new set of lines.

Concrete and pore solution measurements—In this study, in addition to measuring the fresh concrete's electrical resistance, the resistivity of its component pore solution was also available in some cases (low alkali cement). For these, per equations 1 and 2, the ratio of pore solution resistivity to concrete resistance should be directly proportional to the water content of the concrete (w) raised to the power of n . Thus, while a plot of mixture w/cm vs. this resistance ratio only exhibits a weak correlation (Figure 9), the relationship between water content (w) and the ratio is much better defined (Figure 10). In the latter case, a linear fit ($n=1$, parallel model) provides a reasonable fit to the data, as does a power law model with $n=1.17$. This suggests that electrically, the fresh concrete behaves somewhere in between the parallel (e.g., bleed channels) and the 3-D spherical inclusion models shown in Figure 1. Based on the data in Figure 10, either of these models could be applied in a predictive manner to use a measured electrical resistance ratio to predict the water content of the concretes prepared in this study. For the linear model, Monte Carlo-based predictions (generated using R) are shown in the right plot in Figure 10; the average uncertainty in the predicted water content is computed to be about 12.5 lb/yd³ (7.4 kg/m³).

Conclusions and Prospectus

This study has indicated that the electrical resistance of fresh concrete, while not directly related to its w/c or w/cm ratio, can be related in an inverse fashion to volumetric paste (cement+water) content or some multiplicative combination of water and cement contents

$(c \cdot w^n)$ for concretes prepared with a given set of materials. With a concurrent measurement of the pore solution resistivity, it was possible to estimate the water content of the fresh concrete with a computed uncertainty of 12.5 lb/yd³ (7.4 kg/m³), for the low alkali cement concretes examined in this study. Additionally, direct measurement of the pore solution resistivity on corresponding pastes allowed an estimation of w/c with a computed uncertainty of 0.015. The results here have indicated that even when the only variable between different batches is the mixing water content, the precision and the sensitivity of the test needs to be improved further before it can be used as a reliable estimator of mixture proportions (quality control). However, the general insensitivity of the measured electrical resistance values to the presence of the admixtures employed in this study and (hauling) time within 90 min are encouraging results for promoting continued development of the method. Further studies are needed to examine the range of validity of the relationships proposed here, not only in terms of a wider range of mixture proportions but also covering the range of temperatures likely to be encountered in the field during concrete construction. Additionally, possibilities to extract concrete pore solution and measure its electrical resistivity in the field need to be explored.

Acknowledgments

The authors would like to thank NIST summer student volunteers Isaiah Bentz and Lauren Martys for their assistance in preparing the paste mixtures and extracting their pore solutions for the measurement of resistivity, Dr. Adam Pintar of the NIST Statistical Engineering Division for his invaluable assistance with the statistical analysis, and Drs. Timothy Barrett and Kenneth Snyder of NIST and Dr. Robert Spragg of SES Group & Associates for their careful reviews of the manuscript.

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Fig. 1.
Models for relating concrete conductivity (σ) to porosity (Φ).

Fig. 2.
Wireless device and setup to measure electrical resistance of fresh concrete.

Fig. 3.

Measured pore solution resistivity vs. w/c for pastes prepared with low alkali cement; a) the estimated calibration function and b) the estimated analysis function (inverse of the calibration function) with 95% bounds are overlaid. In both, error bars indicate $\pm$ one standard deviation for either 2 ($w/c=0.42$ and 0.47) or 4 ($w/c=0.37$ and 0.535 and $w/c=0.42$ with no HRWRA) repeat measurements on the same solution sample. For b), the 95% bounds represent the uncertainty in the predicted value of w/c for a specified value of pore solution resistivity.

Fig. 4. 42-d compressive strength and fresh concrete electrical resistance vs w/c for Mixtures 1, 2, and 3. 1000 psi is equivalent to 6.89 MPa. Ranges are provided in Table 2.

Fig. 5. 42-d compressive strength and fresh concrete electrical resistance vs w/c for Mixtures 2, 4, 4R, and 5. 1000 psi is equivalent to 6.89 MPa. Ranges are provided in Table 2.

Fig. 6.
 w/c (w/cm) vs. measured fresh concrete resistance for all mixtures.

Fig. 7.

Conductance (inverse of resistance) vs volume of cement, water, or cement and water for all concrete mixtures prepared with the low alkali cement. 1 ft³ = 28.3 L.

Fig. 8.

Resistance vs various combinations of water (w) and cement (c) contents. Cement+water volume is given by $1/(w+c/3.15)$. Independent variable values on the x-axis have been appropriately scaled to cover a similar range on a single plot.

Fig. 9.
 w/cm vs. scaled ratio of pore solution resistivity (ρ_0) to fresh concrete resistance (R).

Fig. 10.

Scaled ratio of pore solution resistivity to fresh concrete resistance vs. water content with a) estimated calibration functions and b) estimated analysis function for the linear model with 95% bounds overlaid. The bounds represent the uncertainty in the predicted value of water content for a specified ratio of pore solution resistivity to fresh concrete resistance. 1 lb/yd³ is equivalent to 0.593 kg/m³.

Table 1

Mixtures Proportions [lbs/yd³ (kg/m³)]

Mixture No.	1	2	3	4	5	6	7	8	9	10
Low Alkali Cement	650 (386)	650 (386)	650 (386)	697 (414)	609 (361)	530 (314)	500 (297)		650 (386)	650 (386)
High Alkali Cement								650 (386)		
Fly Ash							138 (82)			
Water	241 (143)	273 (162)	306 (182)	258 (153)	286 (170)	249 (148)	268 (159)	273 (162)	306 (182)	273 (162)
Target Air, %	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	6.0
HRWRA, oz/cwt	8.50	4.30	2.40	6.50	4.00	5.00	4.00	4.50	0.00	3.00
w/cm or w/cm	0.37	0.42	0.47	0.37	0.47	0.47	0.42	0.42	0.47	0.42
Paste Volume Fraction (w+c) or (w+cm)	0.265	0.285	0.304	0.285	0.285	0.248	0.285	0.285	0.304	0.285

Table 2

Mixture Proportions and Test Results

Mixture ID	1	2	3	4	4R	5	6	7	8	9	10
Yield Adjusted Proportions											
Total Cementitious, lb/yd ³ (kg/m ³)	638 (379)	648 (384)	649 (385)	693 (411)	704 (418)	606 (360)	527 (313)	641 (380)	648 (384)	651 (386)	663 (393)
Low Alkali Portland Cement, lb/yd ³	638 (379)	648 (384)	649 (385)	693 (411)	704 (418)	606 (360)	527 (313)	503 (298)	648 (384)	651 (386)	663 (393)
High Alkali Portland Cement, lb/yd ³								138 (82)			
Fly Ash, lb/yd ³											
Coarse Agg. (No.57), lb/yd ³	1807 (1072)	1836 (1089)	1840 (1092)	1831 (1086)	1859 (1103)	1834 (1088)	1830 (1086)	1852 (1099)	1836 (1089)	1843 (1093)	1879 (1115)
Fine Aggregate, lb/yd ³	1532 (909)	1437 (853)	1355 (804)	1434 (851)	1456 (864)	1436 (852)	1594 (946)	1437 (853)	1437 (853)	1358 (806)	1291 (766)
Mixing Water, lb/yd ³	237 (141)	272 (161)	305 (181)	267 (158)	260 (154)	285 (169)	247 (147)	269 (160)	272 (161)	306 (182)	278 (165)
HRWRA, oz/cwt	8.65	4.30	2.48	6.50	8.50	1.87	5.00	5.09	4.50	0.00	3.00
w/c or w/cm	0.3713	0.42	0.47	0.3849	0.37	0.47	0.47	0.42	0.42	0.47	0.42
Paste Volume (V _{w+cm})	0.261	0.284	0.304	0.289	0.287	0.283	0.246	0.286	0.284	0.304	0.290
Fresh Concrete Properties											
ASTM C143, Slump, in. (mm)	8¼ (210)	3 ¾ (95)	5¼ (133)	8 (203)	8 (203)	4¾ (108)	3 (76)	5 (127)	5 ¼ (133)	3 ½ (89)	1 ¾ (44)
ASTM C138, Density, lb/ft ³ (kg/m ³)	156.1 (2500)	155.3 (2488)	153.7 (2462)	156.5 (2507)	158.5 (2539)	154.1 (2468)	155.5 (2491)	155.5 (2491)	155.3 (2488)	154.0 (2467)	152.3 (2440)
ASTM C138, Gravimetric Air, %	3.8	2.3	2.1	2.6	1.1	2.4	2.6	1.4	2.3	1.9	4.1
ASTM C231, Pressure Air, %	2.9	2.5	2.4	1.8	1.6	2.4	2.8	1.9	2.1	2.0	4.2
ASTM C1064, Temperature, °F (°C)	72 (22)	72 (22)	72 (22)	70 (21)	70 (21)	70 (21)	70 (21)	70 (21)	70 (21)	70 (21)	70 (21)
Resistance Measurement											
Average resistance, Ω	39.3	36.8	33.4	35.0	37.7	37.7	47.3	46.0	24.0	36.0	36.0
Range, Ω	4.0	2.0	3.0	0.0	4.0	3.0	2.0	3.0	5.0	0.0	0.0

Mixture ID	1	2	3	4	4R	5	6	7	8	9	10
90 min Reading, Ω	38.0	36.0	32.0								
After 90 min Lab Mixing, Ω	44.0	38.0	32.3								
42d Strength (ASTM C39)											
Strength, psi (MPa)	11250 (77.6)	9300 (64.1)	7470 (51.5)	10200 (70.3)	11460 (79.0)	7010 (48.3)	8370 (57.7)	8320 (57.4)	7540 (52.0)	6470 (44.6)	6660 (45.9)